STRUCTURAL NOTES

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, 8th EDITION (FBC2023). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ROOF:
- LIVE LOAD - 20 psf.
- DEAD LOAD - 25 psf.
- DEAD LOAD - 5 psf. (AVAILABLE TO RESIST UPLIFT)

WIND: ASCE 7-22
- ULTIMATE WIND SPEED - 182 mph
- ALLOWABLE WIND SPEED - 141 mph
- EXPOSURE C
- ENCLOSED STRUCTURE
- RISK FACTOR IV (FOR ALL ESSENTIAL COMPONENTS; EX: MAIN BUILDING STRUCTURE, GENERATOR/FUEL POD)

WIND: ASCE 7-22
- ULTIMATE WIND SPEED - 161 mph
- ALLOWABLE WIND SPEED - 125 mph
- EXPOSURE C
- ENCLOSED STRUCTURE
- RISK FACTOR II (FOR ALL NON ESSENTIAL COMPONENTS; EX: PRIVACY WALL, DUMPSTER ENCLOSURE)

TORNADO: ASCE 7-22
- TORNADO SPEED - 73 MPH
- EFFECTIVE PLAN AREA - 40,000 FT^2
- EXPOSURE C
- ENCLOSED STRUCTURE
- RISK FACTOR IV

NOTE: THE DESIGN FOR TORNADO LOADS DOES NOT GOVERN FOR THIS STRUCTURE PER THE GUIDELINES OF CHPATER 32 OF ASCE 7-22

SHOP DRAWING REVIEW:

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED.

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER.

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO:
CONCRETE MIX DESIGNS
MASONRY BLOCK
MASONRY BLOCK ACCESSORIES
MASONRY REINFORCING
CONCRETE REINFORCEMENT
STRUCTURAL STEEL (INCLUDING ANCHOR BOLTS)
PRE-ENGINEERED ALUMINUM FRAMING
STEEL BAR JOISTS
METAL ROOF DECK
ANY ALTERNATE MATERIAL/PRODUCT SUBSTITUTIONS

FOUNDATIONS:

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 psf ON COMPACTED FILL OR NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

FORMWORK AND SHORING:

NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO_THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER.

REINFORCING STEEL:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

WELDED WIRE FABRIC:

TO CONFORM TO ASTM A1064, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

CONCRETE:

SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 psi FOR FOUNDATIONS AND SLABS ON GRADE.
4000 psi FOR ALL OTHER STRUCTURAL CONCRETE.
5000 psi FOR TIEBEAMS AS NOTED ON BEAM SCHEDULE AND DRIVEWAY/APPARATUS BAY SLABS ON GRADE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS.

SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1_1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATNG WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

ALL CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK_UP DATA AS PER CHAPTER 26 OF ACI 318_14 AND ACI 214R.

WATER/CEMENT RATIO FOR CONCRETE AT EXTERIOR BALCONIES SHALL NOT EXCEED 0.40 BY WEIGHT AND HAVE 5,000psi MINIMUM COMPRESSIVE CAPACITY.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS -	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS -	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER -	2"
	#5 BARS AND SMALLER -	1 1/2"
WHERE NOT EXPOSED TO EARTH OR WEATHER:		
SLABS, WALLS, AND JOISTS:	#14 & #18 BARS -	1 1/2"
	#11 BARS AND SMALLER -	3/4"
BEAMS AND COLUMNS:	ALL BARS -	1 1/2"

SEE ACI 318 FOR ADDITIONAL REQUIREMENTS AND MORE INFORMATION.
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CONCRETE TESTING: AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST IN PLACE CONCRETE:

A) ASTM C143 _ "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTISIZER.
B) ASTM C39 _ "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:
1 AT 7 DAYS
2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(s) MAY BE DISCARDED.

NON-SHRINK GROUT:

NON-SHRINK GROUT SHALL BE A HIGH-STRENGTH MORTAR OR GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 psi AT 28 DAYS. THE GROUT IS TO BE NON-METALLIC, NON-CORROSIVE, CEMENT-BASED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM C1107. IT SHALL BOND PERMANENTLY TO A CLEAN METAL BASEPLATE AND CONCRETE SUBSTRATE AND WILL NOT SHRINK IN ITS PLASTIC STATE, AS TESTED IN ACCORDANCE WITH ASTM C827.

CHEMICAL ANCHORS:

SHALL BE AN EQUAL TWO PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE", HILTI HIT-HY 150 MAX-SD OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

MASONRY WALLS:

MASONRY UNITS SHALL MEET ASTM C_90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 psi ON THE NET AREA (f'm = 1500 psi). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C_270. GROUT SHALL BE 3000 psi MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C_476. PROVIDE HOOKED DOWELS IN FOOTINGS FOR ALL VERTICAL REINFORCING ABOVE. LAP SPLICES 48 BAR DIAMETERS.

BLOCK CELLS AS SHOWN ON PLANS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS. SEE PLAN NOTES FOR BAR SIZE AND SPACING. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED.

PROVIDE 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR_O_WALL OR ENGINEER APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES, BEGINNING 8" ABOVE FOOTINGS AND FLOOR LEVELS.

GROUT LIFT: AN INCREMENT OF GROUT HEIGHT WITHIN A TOTAL GROUT POUR.
GROUT POUR: THE TOTAL HEIGHT OF MASONRY TO BE GROUTED PRIOR TO ERECTION OF ADDITIONAL MASONRY. A GROUT POUR CONSISTS OF ONE OR MORE GROUT LIFTS. GROUT POURS SHALL SET FOR A MINIMUM OF 4 HOURS BEFORE ANY ADDITIONAL GROUT PLACEMENT.

GROUT SHALL HAVE A SLUMP BETWEEN 8 AND 11 INCHES, EXCEPT SELF-CONSOLIDATING GROUT. JOB-SITE PROPORTIONING OF SELF-CONSOLIDATING GROUT IS NOT PERMITTED.

MASONRY GROUTING REQUIREMENTS:
1. FIELD-MIXED GROUT SHALL BE PLACED WITHIN 1-1/2 HOURS FROM INTRODUCING WATER INTO THE MIXTURE AND BEFORE INITIAL SET.
2. GROUT SLUMP REQUIREMENTS:
   a. FOR GROUT SLUMP BETWEEN 8 AND 10 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 5 FEET.
   b. FOR GROUT SLUMP BETWEEN 10 AND 11 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 12.67 FEET.
   c. FOR SELF-CONSOLIDATING GROUT, THE GROUT LIFT HEIGHT SHALL NOT EXCEED THE GROUT POUR HEIGHT (24 FEET MAX.)
3. GROUT LIFT HEIGHTS EXCEEDING 5 FEET SHALL MEET THE FOLLOWING REQUIREMENTS:
   a. MASONRY MORTAR HAS CURED FOR AT LEAST 4 HOURS.
   b. GROUT SLUMP IS BETWEEN 10 AND 11 INCHES.
   c. NO INTERMEDIATE BOND BEAMS ARE PLACED BETWEEN THE TOP AND BOTTOM OF THE GROUT LIFT HEIGHT.
4. EACH GROUT LIFT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION AT THE TIME OF PLACEMENT. CONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
5. EACH GROUT LIFT SHALL BE RECONSOLIDATED BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED, AND BEFORE ADDING THE SUBSEQUENT GROUT LIFT. RECONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
6. THE TIME BETWEEN PLACING GROUT LIFTS SHALL NOT EXCEED 1 HOUR.
7. THE MAXIMUM POUR HEIGHT IS 24 FEET.
8. A GROUT KEY SHALL BE PROVIDED AT THE TOP OF EACH GROUT LIFT AND GROUT POUR. GROUT KEYS SHOULD BE FORMED BY TERMINATING THE GROUT 1-1/2 INCHES BELOW A MORTAR JOINT.

TIE BEAMS:

BEAMS WITH THE PREFIX "TB" SHALL BE OF CONCRETE POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINFORCING SHALL BE CONTINUOUS THROUGH TIE BEAMS WITH MINIMUM LAP SPLICES OF 48 BAR DIAMETERS AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE TO AREA REQUIRED, IN ACCORDANCE WITH ACI 530.1, SECTION 4.3.3.3 (SOLID METAL OR FELT CAVITY CAPS ARE PROHIBITED).

TIE COLUMNS:

CONCRETE TIE COLUMNS SHALL BE PLACED AFTER THE MASONRY CMU WALLS. THE CONCRETE BLOCK FACING THE TIE COLUMN SHALL BE REMOVED SO THAT WHEN THE CONCRETE TIE COLUMN IS PLACED, THE CONCRETE WILL FLOW INTO THE BLOCK CELL INTERLOCKING THE TIE COLUMN WITH THE BLOCK. THIS SHALL OCCUR AT THE TOP AND BOTTOM OF THE WALL AND AT 24" ON CENTER FOR THE FULL HEIGHT OF THE INTERFACE BETWEEN THE BLOCK AND THE TIE COLUMN.

LINTELS:

MASONRY OPENINGS LESS THAN 6 FEET SHALL BE SPANNED WITH AN 8" SPAN RATED PRECAST/PRESTRESSED CONCRETE LINTEL. ALL PRECAST LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END ON A GROUT FILLED CELL.

MASONRY OPENINGS 6 FEET OR GREATER SHALL BE SPANNED WITH AN 8" SPAN RATED PRECAST/PRESTRESSED CONCRETE LINTEL WITH 1#5 BAR CONTINUOUS. PRECAST LINTEL AND ALL CELLS ABOVE, TO THE BOTTOM OF THE TIE BEAM OR BOND BEAM, SHALL BE GROUTED SOLID. ALL PRECAST LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END ON A GROUT FILLED CELL.

WHERE A CONCRETE COLUMN OR CONCRETE TIE COLUMN IS WITHIN 8" OF A MASONRY OPENING, THE LINTEL SHALL BE AN 8"x16" CONCRETE CAST-IN-PLACE BEAM WITH (2) #5 BARS TOP AND BOTTOM, AND #3 STIRRUPS AT 18" ON CENTER.

STRUCTURAL STEEL:

WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-992 WITH A MINIMUM YIELD STRENGTH OF 50 KSI. STEEL SECTIONS EXCEPT WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-36 WITH A 36 KSI YIELD STRENGTH. ALL STEEL SHALL CONFORM TO THE "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. ALL SHOP CONNECTIONS TO BE WELDED (UTILIZING E70XX ELECTRODES) AND FIELD CONNECTIONS TO BE BOLTED, UNLESS OTHERWISE NOTED ON STRUCTURAL DRAWINGS. STEEL TO RECEIVE ONE SHOP COAT AND ONE FIELD TOUCH UP COAT OF APPROVED PAINT, EXCEPT WHERE GALVANIZING IS INDICATED ON THE DRAWINGS.

STRUCTURAL TUBING SHALL CONFORM TO ASTM A_500, GRADE B, Fy = 46 ksi. STRUCTURAL PIPE SHALL CONFORM TO ASTM A_53 GRADE B, TYPE E OR S; Fy = 35 ksi. WHERE CONCRETE-FILLED COLUMNS ARE SPECIFIED, TUBE & PIPE COLUMNS SHALL BE CONCRETE FILLED IN THE SHOP OR, WHEN THEY SUPPORT CONCRETE BEAMS OR FORMED CONCRETE SLABS, A HOLE PROVIDED IN THE CAP PLATE FOR FIELD FILLING.

ALL BOLTED CONNECTIONS SHALL CONSIST OF MINIMUM 3/4 INCH DIAMETER ASTM A_325 HIGH STRENGTH BOLTS. BEAM CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR FOR THE REACTIONS SHOWN ON THE PLANS. IF NOT SHOWN, THE FABRICATOR SHALL DESIGN THE BEAM CONNECTIONS TO SUPPORT AN END REACTION OF W/2 KIPS FROM TABLE 3-6 "MAXIMUM TOTAL UNIFORM LOAD" OF STEEL CONSTRUCTION MANUAL (13th EDITION), BUT CONNECTIONS SHALL NOT HAVE LESS THAN 2 ROWS OF BOLTS.

MINIMUM CONNECTIONS: AT A MINIMUM THE BOLT ROWS (AT 3" CENTERS) SHALL EXTEND FOR 2/3 OF THE BEAM DEPTH AND AS REQUIRED ABOVE.

SLOTTED HOLES: WHEREVER LONG SLOTTED HOLES ARE USED 5/16" THICK WASHERS SHALL BE PROVIDED PER AISC REQUIREMENTS.

ANCHOR BOLTS:

SHALL CONFORM TO ASTM A_307 OR F1554 (THREADED ROD). ANCHOR BOLTS SHALL BE MINIMUM ASTM A_36 (GRADE 36) STEEL.

MACHINE AND LAG BOLTS:

SHALL BE A_307 HOT DIPPED GALVANIZED WITH GALVANIZED WASHERS.

STEEL JOISTS:

SHALL BE THE SIZE AND SPACING AS SHOWN ON THE STRUCTURAL DRAWINGS AND SHALL BE DESIGNED, FABRICATED, INSTALLED AND BRIDGED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SPECIFICATIONS. ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THERETO AT TOP AND BOTTOM CHORDS. MINIMUM JOIST BRIDGING TERMINATION CONNECTIONS TO MASONRY SHALL BE L3 X 3 X 1/4 X 3" LONG WITH (1) 1/2" DIAMETER ANCHOR BOLT OR 1/4 X 4 X 4 WITH (1) 1/2" X 5" HEADED STUD TO CONCRETE. BRIDGING SHALL BE WELDED OR BOLTED AT ALL POINTS OF CONTACT. WELD SHALL NOT DAMAGE THE JOIST. CROSS BRIDGING SHALL BE WELDED OR BOLTED AT ITS CENTER POINT.

K_SERIES JOISTS SHALL BEAR A MINIMUM OF 2_1/2" ON STEEL BEAMS AND 4" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 5/8" X 4" X 7_1/2" WITH (2) 5/8" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 5/8" X 7_1/2" X 7_1/2" WITH (2) 5/8" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATES WITH A MINIMUM OF (2) 3/16 X 1" FILLET WELDS, UNLESS NOTED OTHERWISE.

LH_SERIES JOISTS SHALL BEAR A MINIMUM OF 4" ON STEEL BEAMS AND 6" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 5/8" X 6" X 12" WITH (3) 5/8" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 5/8" X 9" X 11_5/8" WITH (4) 5/8" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATE WITH A MINIMUM OF (2) 1/4 X 2" FILLET WELDS, UNLESS NOTED OTHERWISE. BACK TO BACK JOISTS SHALL BE OFFSET IF CONCRETE BEAM IS LESS THAN 12" NOMINAL WIDTH OR STEEL BEAM IS LESS THAN 8" WIDE.

SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWING SUBMITTAL SHALL INCLUDE LAYOUT, COMPONENT DESIGNATION, BRIDGING, AND PERTINENT SECTIONS AND DETAILS. SUBMITTALS FOR JOISTS, OTHER THAN STANDARD SJI CATALOG SELECTIONS WHICH HAVE BEEN CHECKED BY SJI, SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

JOIST BEARING: WHERE STEEL BEAMS SUPPORT JOISTS FRAMING FROM ONE SIDE ONLY, OR WHERE JOISTS FROM ONE SIDE ARE 30% LONGER THAN THE JOISTS ON THE OPPOSITE OF THE BEAM, JOISTS SHALL BEAR FULL WIDTH OF THE STEEL BEAM OR S_1/2", WHICH EVER IS LESS.

NET UPLIFT DESIGN: STEEL ROOF JOISTS SHALL BE DESIGNED TO WITHSTAND THE NET UPLIFT DUE TO WIND AS REQUIRED ASCE 7, NOT LESS THAN ZONE 2 WIND PRESSURES AS SHOWN IN COMPONENT & CLADDING SCHEDULE. IN ADDITION TO THE STANDARD SJI BOTTOM CHORD BRIDGING, WHICH INCLUDES THE FIRST END PANELS, THE JOIST MANUFACTURER SHALL PROVIDE DESIGN CALCULATIONS FOR UPLIFT, EITHER CONFIRMING THE SJI BRIDGING REQUIREMENT OR PROVIDING A DESIGN ADEQUATE FOR THE UPLIFT. THIS SUBMITTAL SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

STEEL JOIST GIRDERS: SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR JOIST GIRDERS AS PUBLISHED BY THE STEEL JOIST INSTITUTE (SJI). JOIST GIRDERS SHALL BE OF THE DEPTH AND SPACING SHOWN ON THE STRUCTURAL DRAWINGS, AND UNLESS OTHERWISE NOTED, JOIST GIRDERS SHALL BE DESIGNED AS SIMPLY SUPPORTED PRIMARY MEMBERS PROPORTIONED SUCH THAT THEY MAY BE ERECTED WITHOUT BRIDGING. JOIST GIRDER PANEL POINTS SHALL BE ALIGNED WITH SECONDARY MEMBERS.

CONCRETE SUPPORTS: UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS, JOIST GIRDERS SHALL BEAR ON 5/8" x 7" x 1'_0" STEEL BEARING PLATES WITH A MINIMUM OF FOUR 1/2" DIAMETER x 6" LONG SHEAR STUD CONNECTORS (CONFORMING TO AWS D1.1). THE CONNECTION TO THE BEARING PLATE SHALL BE DESIGNED TO RESIST ALL UPLIFT AND SHEAR LOADS; HOWEVER, A MINIMUM OF TWO 1/4" FILLET WELDS 2 INCHES LONG SHALL BE UTILIZED. MINIMUM JOIST GIRDER BEARING SHALL BE 6".

STEEL SUPPORTS: UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS, JOIST GIRDERS MAY BE DIRECTLY CONNECTED TO STEEL GIRDERS HAVING A FLANGE THICKNESS GREATER THAN 3/8". THE CONNECTION SHALL BE DESIGNED TO RESIST ALL UPLIFT AND SHEAR LOADS; HOWEVER, A MINIMUM OF TWO 1/4" FILLET WELDS 2 INCHES LONG OR TWO 3/4" DIAMETER HIGH STRENGTH BOLTS SHALL BE UTILIZED. MINIMUM JOIST GIRDER BEARING SHALL BE 4 INCHES.

SUPPLIER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION. SUBMITTALS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

STEEL ROOF DECK:

SHALL BE 1 1/2" DEEP, 22 GAGE GALVANIZED (G60) CORRUGATED STEEL DECK, AND SHALL CONFORM TO PROVISIONS OF THE STEEL DECK INSTITUTE (SDI). FASTEN DECK TO STRUCTURAL STEEL SUPPORTS BY PUDDLE WELDS IN A 36/4 PATTERN. USE WELDING WASHERS FOR DECK THINNER THAN 22 GAGE. FASTEN SIDE LAPS AT MID_SPAN WITH (1) #10 SCREW. (UNLESS OTHERWISE INDICATED ON THE PLANS)

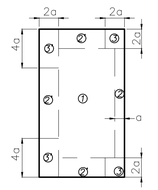
WIND LOAD SCHEDULE

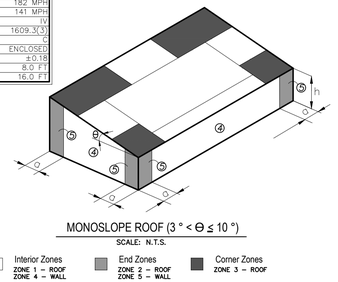
SCHEDULE OF COMPONENTS AND CLADDING LOADS

ZONE	ZONE DESCRIPTION	TRIBUTARY AREA (SF)	IN (PRESSURE) (+ PSF)	OUT (PRESSURE) (- PSF)
1	ROOF INTERIOR ZONE	LESS THAN 20	18.3	48.7
		20 — 100	17.5	48.7
		MORE THAN 100	14.5	48.7
2	ROOF, EDGE ZONE	LESS THAN 20	18.3	56.3
		20 — 100	17.5	54.4
		MORE THAN 100	14.5	52.5
2'	ROOF, EDGE ZONE	LESS THAN 20	18.3	67.7
		20 — 100	17.5	67.0
		MORE THAN 100	14.5	63.9
3	ROOF, CORNER ZONE	LESS THAN 20	18.3	75.4
		20 — 100	17.5	68.5
		MORE THAN 100	14.5	52.5
3'	ROOF, CORNER ZONE	LESS THAN 20	18.3	105.8
		20 — 100	17.5	94.4
		MORE THAN 100	14.5	67.7
4	WALL, INTERIOR ZONE	LESS THAN 20	44.9	48.7
		20 — 100	43.0	46.8
		MORE THAN 100	38.1	43.0
5	WALL, EDGE ZONE	LESS THAN 20	44.9	60.1
		20 — 100	43.0	56.3
		MORE THAN 100	38.1	46.8

NOTE: WIND PRESSURES SHOWN ARE BASED ON Vasd

CODE =	ASCE 7-22
ULTIMATE WIND SPEED Vult =	182 MPH
ALLOWABLE WIND SPEED Vasd =	141 MPH
RISK CATEGORY =	IV
WIND SPEED MAP =	1609.3(3)
EXPOSURE =	C
ENCLOSURE CLASSIFICATION =	ENCLOSED
INTERNAL PRESSURE COEFFICIENT (GCpi) =	±0.18
a =	8.0 FT
2a =	16.0 FT





MONOSLOPE ROOF (3 ° < Θ ≤ 10 °)

SCALE: N.T.S.

Interior Zones — ZONE 1 — ROOF, ZONE 4 — WALL
End Zones — ZONE 2 — ROOF, ZONE 5 — WALL
Corner Zones — ZONE 3 — ROOF

Anthony R. Wilson, P.E.

Digitally signed by Anthony R. Wilson, P.E.
DN: E=tony@wilsonstructural.com, CN=ANTHONY R. WILSON, O=WILSON STRUCTURAL CONSULTING ENGINEERS, LLC
SERIALNUMBER=IA7L20250609403052, L=Sarasota, S=Florida, C=US
Date: 2025.09.01 16:22:27-04'00'

This drawing has been electronically signed and sealed by Anthony R. Wilson, P.E. on 10/01/2025 using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on an electronic device.

ANTHONY R. WILSON LICENSE No. 79451 STATE OF FLORIDA PROFESSIONAL ENGINEER
10/1/2025
ENGINEER OF RECORD
ANTHONY R. WILSON
FL P.E. # 79451

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS WILL COMPLY WITH ALL APPLICABLE MINIMUM BUILDING CODES.
FILE: 24DW-0502
FIRM REGISTRY #36167

No.	Description	Date

Project #
Date: 30 September, 2025
Drawn: HP
Checked: JK
Scale 1/8 =1"

North Collier Fire Control & Rescue District Station 49

COCOHATCHEE ROAD NAPLES FLORIDA 34110
FOR: NORTH COLLIER FIRE RESCUE DISTRICT

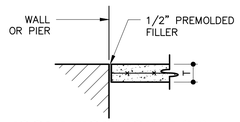
STRUCTURAL NOTES

Lars W Young Inc

Florida Registration # AR0016755

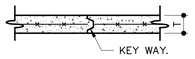
Architecture
6961 Sable Ridge Ln
Naples, FL 34109
phone: 239.591.8899
lwyarch@gmail.com

S1.0



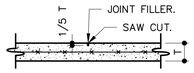
ISOLATION JOINT (IJ)

SCALE: NTS



CONSTRUCTION JOINT (KJ)

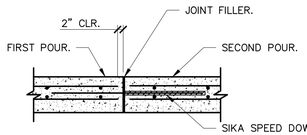
SCALE: NTS



CONTROL JOINT (CJ)

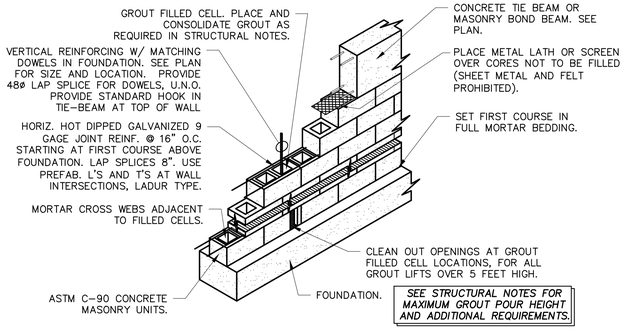
SCALE: NTS

SAW CUT AS SOON AS POSSIBLE WITHOUT RAVELING CONCRETE. (4 TO 16 HOURS AFTER POUR).



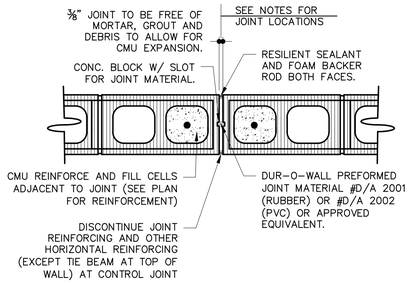
MOVEABLE JOINT (MJ) - 8" SLAB

SCALE: NTS



TYPICAL MASONRY WALL CONSTRUCTION

SCALE: NTS

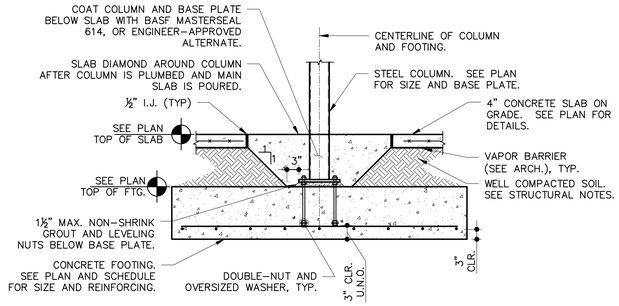


CONTROL JOINT DETAIL MASONRY WALL

SCALE: NTS

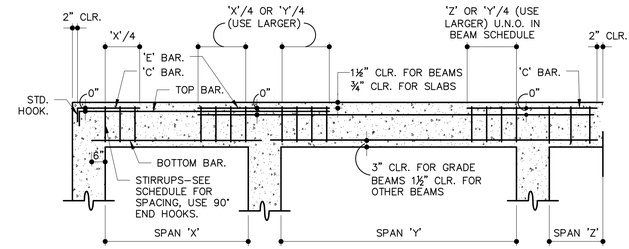
NOTES:

1. COORDINATE LOCATION OF CMU CONTROL JOINTS WITH CONCRETE WALL AND STUCCO CONTROL JOINTS WHERE POSSIBLE.
2. BLOCK CONTROL JOINTS SHALL BE LOCATED AT A SPACING OF NO MORE THAN 25 FEET BETWEEN JOINTS AND NO MORE THAN 12 FEET FROM A BUILDING CORNER.
3. CONTROL JOINTS SHALL BE LOCATED AT CHANGES IN WALL HEIGHTS & CHANGES IN WALL THICKNESS.
4. CONTROL JOINTS ADJACENT TO WALL OPENINGS SHALL BE NO CLOSER THAN 8" TO THE OPENING. DEPENDING ON THE OPENING WIDTH, THE JOINT MAY NEED TO BE FURTHER AWAY SO THAT MULTIPLE REINFORCED CELLS CAN BE LOCATED BETWEEN THE CONTROL JOINT AND THE WALL OPENING. FOR OPENINGS OVER 4 FEET IN WIDTH, CONSULT WITH THE ENGINEER FOR JOINT LOCATIONS NEXT TO WALL OPENINGS
5. SEE THE ARCHITECTURAL DRAWINGS FOR JOINT LOCATIONS. IF THE ARCHITECTURAL DRAWINGS CONFLICT WITH THE ABOVE REQUIREMENTS, THE ARCHITECT AND STRUCTURAL ENGINEER SHALL BE CONSULTED BY THE CONTRACTOR.
6. BLOCK WALL JOINTS THAT LINE UP WITH FLOOR & ROOF EXPANSION JOINTS SHALL BE DETAILED ESPECIALLY TO ALLOW A HIGHER MAGNITUDE OF MOVEMENT THAN THIS CONTROL JOINT DETAIL WILL ALLOW. DO NOT USE THIS DETAIL AT THESE EXPANSION JOINTS.



STEEL COLUMN BASE

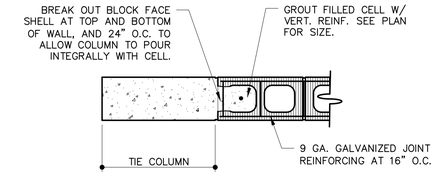
SCALE: NTS



NOTES:
1. START STIRRUP SPACING AT FACE OF SUPPORT.
2. ADD 1#4 LONGITUDINAL WHERE CORNER OF STIRRUP IS VACANT.

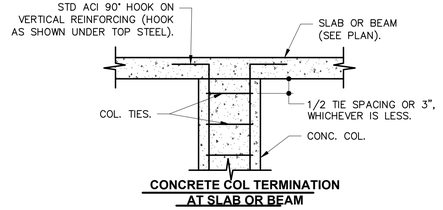
TYPICAL BENDING DIAGRAM FOR BEAMS AND ONE WAY SLABS

SCALE: NTS

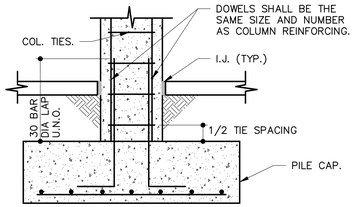


TIE COLUMN REINFORCING

SCALE: NTS



CONCRETE COL TERMINATION AT SLAB OR BEAM



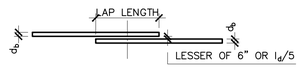
CONCRETE COLUMN BASE

TYPICAL COLUMN REINFORCEMENT DETAIL

SCALE: NTS

BAR SIZE	TENSION LAP / SPLICE LENGTH						COMPRESSION LAP / SPLICE LENGTH
	TOP BARS			OTHER BARS			ALL BARS
	3000 PSI	4000 PSI	5000 PSI	3000 PSI	4000 PSI	5000 PSI	ALL CONCRETE WITH $f'_c \geq$ 3000 psi
#3	28	24	22	22	19	17	12
#4	37	32	29	29	25	22	15
#5	47	40	36	36	31	28	19
#6	56	48	43	43	37	33	23
#7	81	70	63	63	54	49	27
#8	93	80	72	72	62	55	30
#9	105	91	81	81	70	63	34
#10	118	102	91	91	79	70	38

BAR SIZE	HOOKED TENSION DEVELOPMENT LENGTH (l_{dh}) INCHES		
	3000 PSI	4000 PSI	5000 PSI
#3	9	7	7
#4	11	10	9
#5	14	12	11
#6	17	15	13
#7	19	17	15
#8	22	19	17
#9	25	22	19
#10	28	24	22

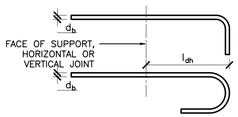


NOTES:

1. REFER TO "HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE" WHEN THE STRAIGHT DEVELOPMENT LENGTH IN TENSION CANNOT BE ACCOMMODATED IN THE CONCRETE SECTION.
2. ALWAYS USE TENSION LAP SPLICE LENGTH VALUES, UNLESS THE PLANS OR DETAILS NOTE OTHERWISE.
3. TABULATED DEVELOPMENT AND LAP SPLICE LENGTHS ARE BASED ON REINFORCING STEEL YIELD STRENGTH F_y=60 KSI, NORMAL WEIGHT CONCRETE, AND CLASS B LAPS.
4. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE. TOP BAR FACTOR DOES NOT APPLY TO BARS IN WALLS.
5. WHEN DIFFERENT BAR DIAMETERS ARE SPLICED, USE SMALLER BAR LAP SPLICE LENGTH.
6. ALL TABULATED VALUES ARE MINIMUM LENGTH, IN CASE OF CONFLICT WITH PLANS, SECTIONS, OR DETAILS USE LONGER LENGTH.
7. d_b = BAR DIAMETER
8. l_d = DEVELOPMENT, LAP OR SPLICE LENGTH.
9. ADJUST TABULATED LENGTH BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT FACTORS ARE CUMULATIVE: (E.G. 1.30x1.50 = 1.95)
   - A. LIGHT WEIGHT CONCRETE: 1.30
   - B. 3 OR LESS BUNDLED BARS: 1.20
   - C. 4 OR MORE BUNDLED BARS: 1.33
   - D. CLEAR SPACING LESS THAN $2d_b$ AND CLEAR COVER LESS THAN d_b 1.50
   - E. CLASS A LAP SPLICE 0.77
   - F. EPOXY COATED BARS 1.50
10. WELDED AND/OR MECHANICAL SPLICES MAY BE USED AT THE GENERAL CONTRACTOR'S OPTION PROVIDED THAT THE SPLICE IS CAPABLE OF DEVELOPING AT LEAST 125% OF THE YIELD STRENGTH OF THE LARGER BAR IN TENSION. WHERE WELDED AND/OR MECHANICAL SPLICES ARE TO BE USED, THE GENERAL CONTRACTOR SHALL SUBMIT FULL DATA OF THE PROPOSED MATERIAL, PROCEDURES, AND INSTALLATION INSTRUCTIONS TO THE ENGINEER FOR REVIEW AS A SHOP DRAWING SUBMISSION.
11. USE MECHANICAL COUPLERS FOR #14 AND LARGER BARS.
12. LAP SPLICES IN CONCRETE MASONRY SHALL BE AS SPECIFIED IN STRUCTURAL NOTES.

TYP. STRAIGHT REINFORCEMENT DEVELOPMENT AND SPLICE LENGTH SCHEDULE

SCALE: NTS

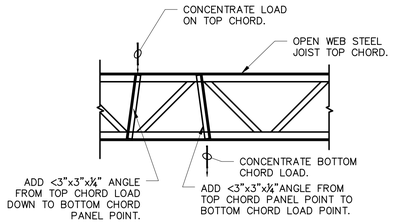


NOTES:

1. SEE TYPICAL TIE AND STIRRUP HOOKS DETAIL FOR ADDITIONAL INFORMATION.
2. TABULATED DEVELOPMENT LENGTH ARE BASED ON REINFORCING STEEL YIELD STRENGTH F_y = 60 KSI AND NORMAL WEIGHT CONCRETE.
3. ALL TABULATED VALUES ARE MINIMUM LENGTHS. IN CASE OF CONFLICT WITH THE PLANS, SECTIONS, OR DETAILS, USE THE LONGER LENGTH.
4. d_b = BAR DIAMETER
5. l_{dh} = DEVELOPMENT LENGTH
6. ADJUST TABULATED LENGTHS BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT THE FACTORS ARE CUMULATIVE.
   - A. REINFORCING BAR STRENGTH OTHER THAN 60 KSI: (F_y/60 KSI)
   - B. LIGHT WEIGHT CONCRETE: 1.30
   - C. EPOXY COATED BARS 1.20

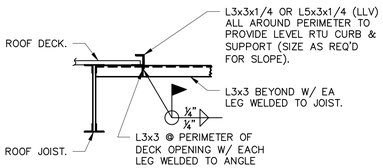
TYP. HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE

SCALE: NTS



TYPICAL JOIST REINFORCEMENT DETAIL AT CONCENTRATED LOADS

SCALE: NTS



DECK OPENING & RTU LEVELING/SUPPORT DETAIL

SCALE: NTS

Lars W Young Inc

ANTHONY R. WILSON LICENSE No. 79451 STATE OF FLORIDA 10/1/2025 PROFESSIONAL ENGINEER
ENGINEER OF RECORD
ANTHONY R. WILSON
FL P.E. # 79451

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES.

FILE: 24DW-0502

FIRM REGISTRY #36167

No.	Description	Date

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Date: 30 September, 2025
Drawn: HP
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Scale 1/8 =1"

North Collier Fire Control & Rescue District Station 49

COCOHATCHEE ROAD NAPLES FLORIDA 34110
FOR: NORTH COLLIER FIRE RESCUE DISTRICT

TYPICAL DETAILS

Florida Registration # AR0016755

Architecture
6961 Sable Ridge Ln
Naples, FL 34109
phone: 239.591.8899
lwyarch@gmail.com

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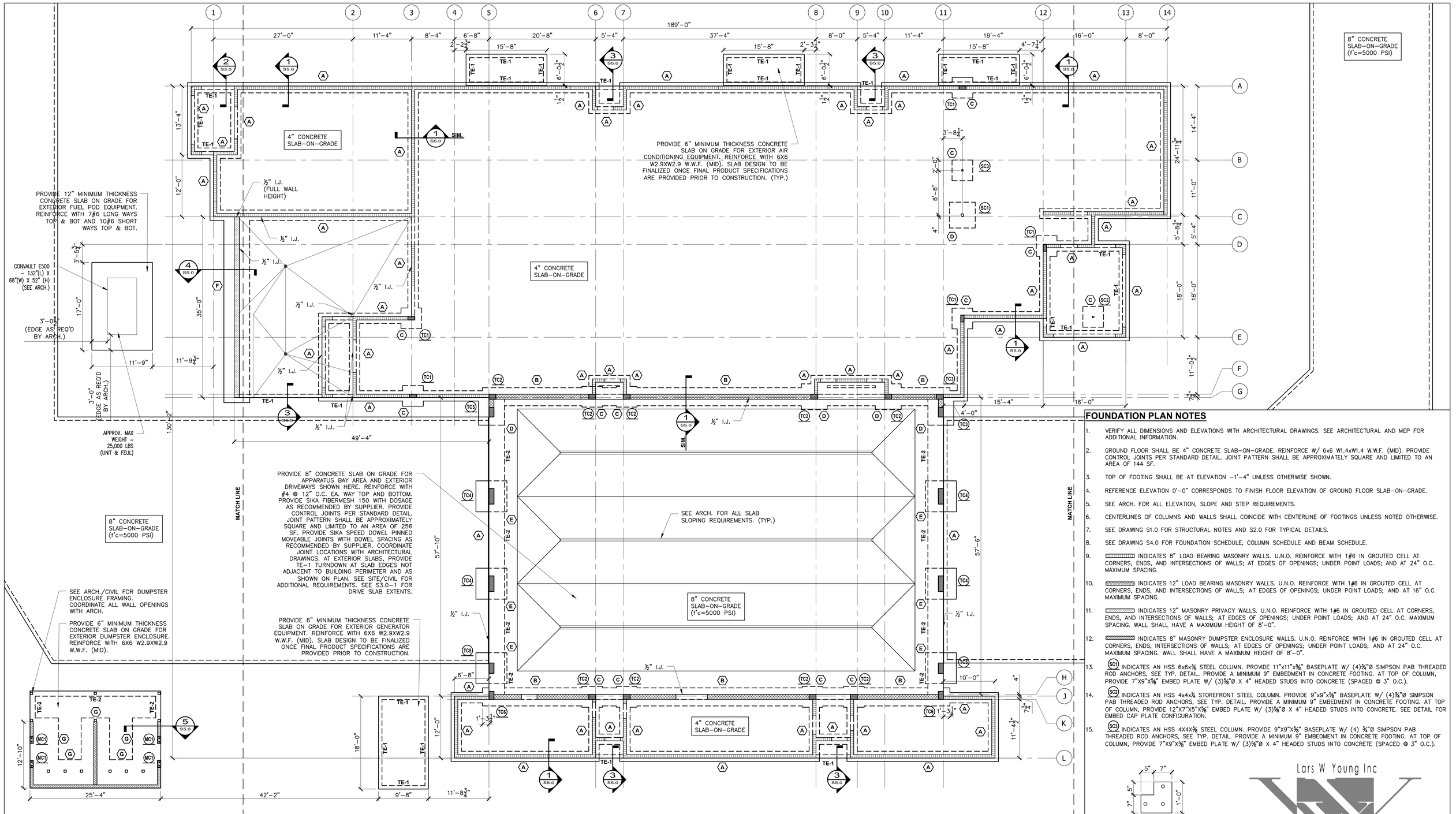
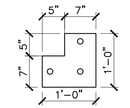


FOUNDATION & FIRST FLOOR PLAN

SCALE: 1/8" = 1'-0"

FOUNDATION PLAN NOTES

1. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
2. GROUND FLOOR SHALL BE 4" CONCRETE SLAB-ON-GRADE. REINFORCE W/ 6x6 W1.4xW1.4 W.W.F. (MID). PROVIDE CONTROL JOINTS PER STANDARD DETAIL. JOINT PATTERN SHALL BE APPROXIMATELY SQUARE AND LIMITED TO AN AREA OF 144 SF.
3. TOP OF FOOTING SHALL BE AT ELEVATION -1'-4" UNLESS OTHERWISE SHOWN.
4. REFERENCE ELEVATION 0'-0" CORRESPONDS TO FINISH FLOOR ELEVATION OF GROUND FLOOR SLAB-ON-GRADE.
5. SEE ARCH. FOR ALL ELEVATION, SLOPE AND STEP REQUIREMENTS.
6. CENTERLINES OF COLUMNS AND WALLS SHALL COINCIDE WITH CENTERLINE OF FOOTINGS UNLESS NOTED OTHERWISE.
7. SEE DRAWING S1.0 FOR STRUCTURAL NOTES AND S2.0 FOR TYPICAL DETAILS.
8. SEE DRAWING S4.0 FOR FOUNDATION SCHEDULE, COLUMN SCHEDULE AND BEAM SCHEDULE.
9. INDICATES 8" LOAD BEARING MASONRY WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, AND INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 24" O.C. MAXIMUM SPACING.
10. INDICATES 12" LOAD BEARING MASONRY WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, AND INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 16" O.C. MAXIMUM SPACING.
11. INDICATES 12" MASONRY PRIVACY WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, AND INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 24" O.C. MAXIMUM SPACING. WALL SHALL HAVE A MAXIMUM HEIGHT OF 8'-0".
12. INDICATES 8" MASONRY DUMPSTER ENCLOSURE WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 24" O.C. MAXIMUM SPACING. WALL SHALL HAVE A MAXIMUM HEIGHT OF 8'-0".
13. (SC1) INDICATES AN HSS 6x6x⅜ STEEL COLUMN. PROVIDE 11"x11"x⅝" BASEPLATE W/ (4)¾"Ø SIMPSON PAB THREADED ROD ANCHORS, SEE TYP. DETAIL. PROVIDE A MINIMUM 9" EMBEDMENT IN CONCRETE FOOTING. AT TOP OF COLUMN, PROVIDE 7"X9"X⅝" EMBED PLATE W/ (3)⅝"Ø X 4" HEADED STUDS INTO CONCRETE (SPACED @ 3" O.C.).
14. (SC2) INDICATES AN HSS 4x4x¼ STOREFRONT STEEL COLUMN. PROVIDE 9"x9"x⅝" BASEPLATE W/ (4)¾"Ø SIMPSON PAB THREADED ROD ANCHORS, SEE TYP. DETAIL. PROVIDE A MINIMUM 9" EMBEDMENT IN CONCRETE FOOTING. AT TOP OF COLUMN, PROVIDE 12"X7"X5"X⅝" EMBED PLATE W/ (3)⅝"Ø X 4" HEADED STUDS INTO CONCRETE. SEE DETAIL FOR EMBED CAP PLATE CONFIGURATION.
15. (SC3) INDICATES AN HSS 4X4X⅜ STEEL COLUMN. PROVIDE 9"X9"X⅝" BASEPLATE W/ (4) ¾"Ø SIMPSON PAB THREADED ROD ANCHORS, SEE TYP. DETAIL. PROVIDE A MINIMUM 9" EMBEDMENT IN CONCRETE FOOTING. AT TOP OF COLUMN, PROVIDE 7"X9"X⅝" EMBED PLATE W/ (3)⅝"Ø X 4" HEADED STUDS INTO CONCRETE (SPACED @ 3" O.C.).



SC2 COLUMN CAP PLATE DETAIL
SCALE:NTS

COLUMN SYMBOLS

- INDICATES COLUMN BELOW
- INDICATES COLUMN THRU
- INDICATES COLUMN ABOVE

STEEL SYMBOLS

BAR SIZE — 4 / 12 T — T=TOP, B=BOTTOM, C=C, E=E
BAR SPACING
BAR PLACEMENT

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FOUNDATION PLAN

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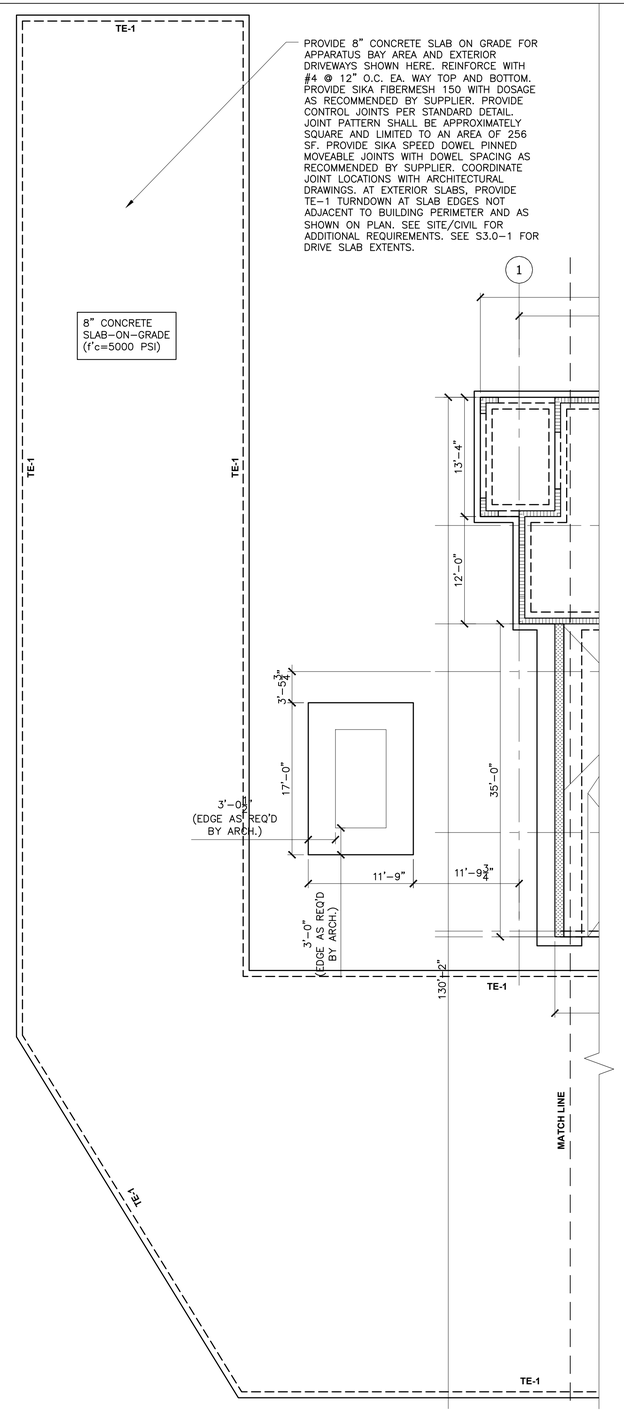
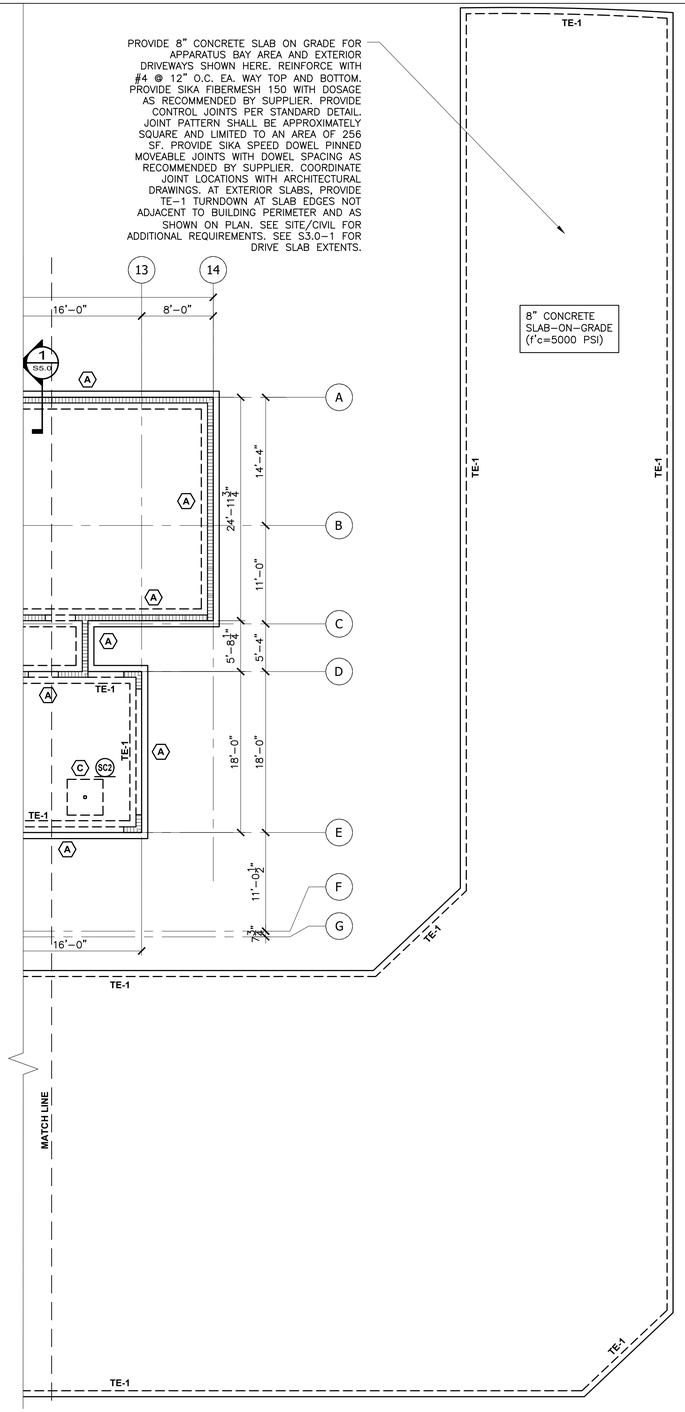
S3.0





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3. TOP OF FOOTING SHALL BE AT ELEVATION -1'-4" UNLESS OTHERWISE SHOWN.
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5. SEE ARCH. FOR ALL ELEVATION, SLOPE AND STEP REQUIREMENTS.
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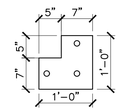
9. INDICATES 8" LOAD BEARING MASONRY WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, AND INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 24" O.C. MAXIMUM SPACING.
10. INDICATES 12" LOAD BEARING MASONRY WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, AND INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 16" O.C. MAXIMUM SPACING.
11. INDICATES 12" MASONRY PRIVACY WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, AND INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 24" O.C. MAXIMUM SPACING. WALL SHALL HAVE A MAXIMUM HEIGHT OF 8'-0".
12. INDICATES 8" MASONRY DUMPSTER ENCLOSURE WALLS. U.N.O. REINFORCE WITH 1#6 IN GROUTED CELL AT CORNERS, ENDS, INTERSECTIONS OF WALLS; AT EDGES OF OPENINGS; UNDER POINT LOADS; AND AT 24" O.C. MAXIMUM SPACING. WALL SHALL HAVE A MAXIMUM HEIGHT OF 8'-0".
13. (SC1) INDICATES AN HSS 6x6x3/8 STEEL COLUMN. PROVIDE 11"x11"x5/8" BASEPLATE W/ (4)3/4"Ø SIMPSON PAB THREADED ROD ANCHORS, SEE TYP. DETAIL. PROVIDE A MINIMUM 9" EMBEDMENT IN CONCRETE FOOTING. AT TOP OF COLUMN, PROVIDE 7"X9"X5/8" EMBED PLATE W/ (3)5/8"Ø X 4" HEADED STUDS INTO CONCRETE (SPACED @ 3" O.C.).
14. (SC2) INDICATES AN HSS 4x4x1/4 STOREFRONT STEEL COLUMN. PROVIDE 9"x9"x5/8" BASEPLATE W/ (4)3/4"Ø SIMPSON PAB THREADED ROD ANCHORS, SEE TYP. DETAIL. PROVIDE A MINIMUM 9" EMBEDMENT IN CONCRETE FOOTING. AT TOP OF COLUMN, PROVIDE 12"X7"X5"X5/8" EMBED PLATE W/ (3)5/8"Ø X 4" HEADED STUDS INTO CONCRETE. SEE DETAIL FOR EMBED CAP PLATE CONFIGURATION.
15. (SC3) INDICATES AN HSS 4X4X3/8 STEEL COLUMN. PROVIDE 9"X9"X5/8" BASEPLATE W/ (4) 3/4"Ø SIMPSON PAB THREADED ROD ANCHORS, SEE TYP. DETAIL. PROVIDE A MINIMUM 9" EMBEDMENT IN CONCRETE FOOTING. AT TOP OF COLUMN, PROVIDE 7"X9"X5/8" EMBED PLATE W/ (3)5/8"Ø X 4" HEADED STUDS INTO CONCRETE (SPACED @ 3" O.C.).



SC2 COLUMN CAP PLATE DETAIL
SCALE:NTS

Lars W Young Inc

PARTIAL FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

COLUMN SYMBOLS

- INDICATES COLUMN BELOW
- INDICATES COLUMN THRU
- INDICATES COLUMN ABOVE

STEEL SYMBOLS

4 / 12 T (T=TOP, B=BOTTOM, C=C, E=E)
BAR SIZE
BAR SPACING
BAR PLACEMENT

STATE OF FLORIDA — ANTHONY R. WILSON — No. 79451 — LICENSE — PROFESSIONAL ENGINEER
10/1/2025
ENGINEER OF RECORD
ANTHONY R. WILSON
FL P.E. # 79451
FILE: 24DW-0502

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THE PLANS, COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FIRM REGISTRY #36167

No.	Description	Date

Project #
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Drawn: HP
Checked: JK
Scale 1/8 =1"

North Collier Fire Control & Rescue District Station 49

COCOHATCHEE ROAD NAPLES FLORIDA 34110
FOR: NORTH COLLIER FIRE RESCUE DISTRICT RTIAL FOUNDATION PLAN

Architecture
6961 Sable Ridge Ln
Naples, FL 34109
phone: 239.591.8899
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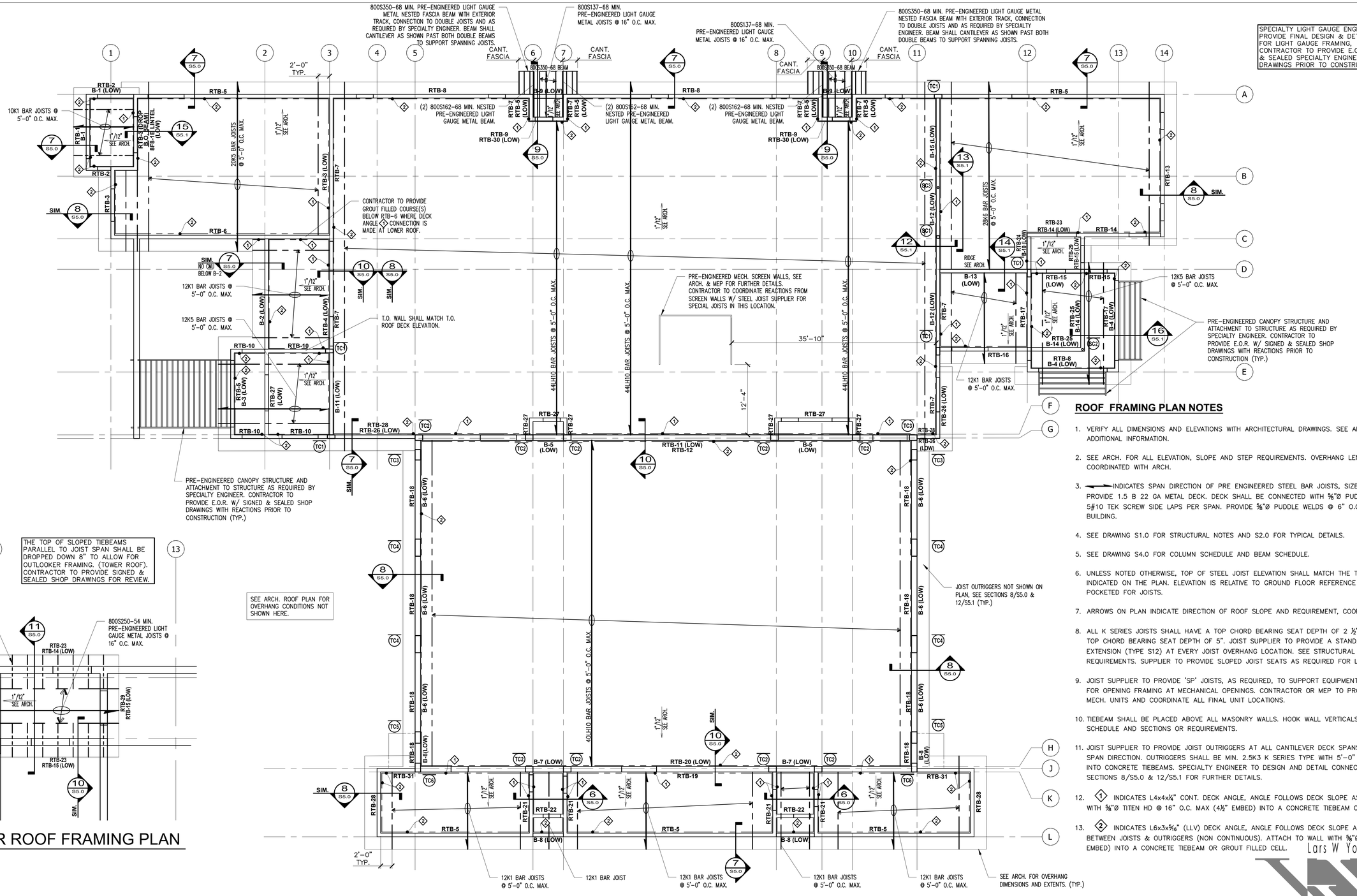
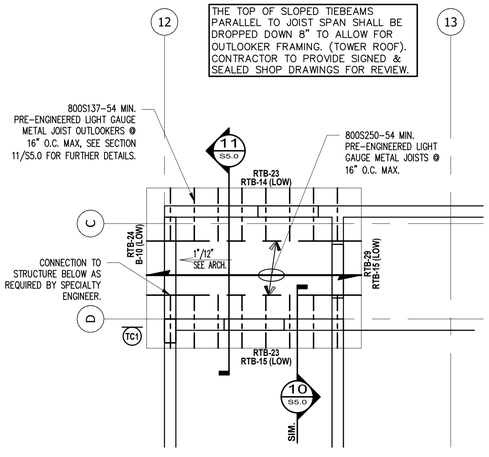
S3.0-1



ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"



TOWER ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

ROOF FRAMING PLAN NOTES

1. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
2. SEE ARCH. FOR ALL ELEVATION, SLOPE AND STEP REQUIREMENTS. OVERHANG LENGTHS AND EXTENTS SHALL BE COORDINATED WITH ARCH.
3. ⟵ INDICATES SPAN DIRECTION OF PRE ENGINEERED STEEL BAR JOISTS, SIZE AND SPACING AS INDICATED. PROVIDE 1.5 B 22 GA METAL DECK. DECK SHALL BE CONNECTED WITH 5/8"Ø PUDDLE WELDS (36/4). PROVIDE 5#10 TEK SCREW SIDE LAPS PER SPAN. PROVIDE 5/8"Ø PUDDLE WELDS @ 6" O.C. AROUND PERIMETER OF BUILDING.
4. SEE DRAWING S1.0 FOR STRUCTURAL NOTES AND S2.0 FOR TYPICAL DETAILS.
5. SEE DRAWING S4.0 FOR COLUMN SCHEDULE AND BEAM SCHEDULE.
6. UNLESS NOTED OTHERWISE, TOP OF STEEL JOIST ELEVATION SHALL MATCH THE TOP OF TIE-BEAM ELEVATION AS INDICATED ON THE PLAN. ELEVATION IS RELATIVE TO GROUND FLOOR REFERENCE ELEVATION. TIEBEAMS SHALL BE POCKETED FOR JOISTS.
7. ARROWS ON PLAN INDICATE DIRECTION OF ROOF SLOPE AND REQUIREMENT, COORDINATE WITH ARCH.
8. ALL K SERIES JOISTS SHALL HAVE A TOP CHORD BEARING SEAT DEPTH OF 2 ½"; ALL LH SERIES SHALL HAVE A TOP CHORD BEARING SEAT DEPTH OF 5". JOIST SUPPLIER TO PROVIDE A STANDARD 'S' TYPE TOP CHORD EXTENSION (TYPE S12) AT EVERY JOIST OVERHANG LOCATION. SEE STRUCTURAL NOTES FOR FURTHER REQUIREMENTS. SUPPLIER TO PROVIDE SLOPED JOIST SEATS AS REQUIRED FOR LEVEL BEARING.
9. JOIST SUPPLIER TO PROVIDE 'SP' JOISTS, AS REQUIRED, TO SUPPORT EQUIPMENT LOADS INDICATED. SEE TYPICAL FOR OPENING FRAMING AT MECHANICAL OPENINGS. CONTRACTOR OR MEP TO PROVIDE ANY ADDITIONAL LOADS FOR MECH. UNITS AND COORDINATE ALL FINAL UNIT LOCATIONS.
10. TIEBEAM SHALL BE PLACED ABOVE ALL MASONRY WALLS. HOOK WALL VERTICALS INTO TIE-BEAM. SEE PLAN, SCHEDULE AND SECTIONS OR REQUIREMENTS.
11. JOIST SUPPLIER TO PROVIDE JOIST OUTRIGGERS AT ALL CANTILEVER DECK SPANS, PERPENDICULAR TO MAIN JOIST SPAN DIRECTION. OUTRIGGERS SHALL BE MIN. 2.5K3 K SERIES TYPE WITH 5'-0" O.C. MAX SPACING AND POCKETED INTO CONCRETE TIEBEAMS. SPECIALTY ENGINEER TO DESIGN AND DETAIL CONNECTION TO MAIN ROOF JOISTS. SEE SECTIONS 8/S5.0 & 12/S5.1 FOR FURTHER DETAILS.
12. ◇1 INDICATES L4x4x¼" CONT. DECK ANGLE, ANGLE FOLLOWS DECK SLOPE AS REQUIRED. ATTACH TO WALL WITH 3/8"Ø TITEN HD @ 16" O.C. MAX (4½" EMBED) INTO A CONCRETE TIEBEAM OR GROUT FILLED CELL.
13. ◇2 INDICATES L6x3x5/16" (LLV) DECK ANGLE, ANGLE FOLLOWS DECK SLOPE AS REQUIRED. ANGLE SHALL SPAN BETWEEN JOISTS & OUTRIGGERS (NON CONTINUOUS). ATTACH TO WALL WITH 5/8"Ø TITEN HD @ 16" O.C. MAX (4½" EMBED) INTO A CONCRETE TIEBEAM OR GROUT FILLED CELL.

COLUMN SYMBOLS

- ○ – INDICATES COLUMN BELOW
- ○ – INDICATES COLUMN THRU
- ○ – INDICATES COLUMN ABOVE

STEEL SYMBOLS

4 / 12 T (T=TOP, B=BOTTOM, C=CJ, E=E)

BAR SIZE
BAR SPACING
BAR PLACEMENT

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FIRM REGISTRY #36167

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North Collier Fire Control & Rescue District Station 49

COCOHATCHEE ROAD NAPLES FLORIDA 34110
FOR: NORTH COLLIER FIRE RESCUE DISTRICT

ROOF FRAMING PLAN

Lars W Young Inc
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Naples, FL 34109
phone: 239.591.8899
Florida Registration # AR0016755
lwyarch@gmail.com

S3.1

FOOTING SCHEDULE

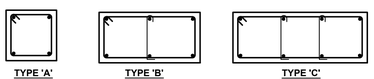
MARK	SIZE W x L	DEPTH	REINFORCING	REMARK
A	2'–0"xCONT.	12"	2#5 CONT. BOT.	
B	3'–6"xCONT.	12"	3#5 CONT. BOT.	
C	4'–0"x4'–0	12"	4#5 BOT. EA. WAY	
D	5'–0"x5'–0"	12"	5#5 BOT. EA. WAY	
E	6'–0"x6'–0"	14"	5#6 BOT. EA WAY	
F	5'–0"xCONT.	12"	5#5 CONT. BOT. #5 TRANSVERSE TOP @ 24" O.C.	T.O. FTG= –1'–4"
G	5'–6"xCONT.	12"	5#5 CONT. BOT. & 2#5 CONT. TOP #5 TRANSVERSE TOP @ 16" O.C.	T.O. FTG=T.O. SLAB
TE-1	0'–8"xCONT.	12"	1#5 CONT. BOT.	THICKENED SLAB EDGE
TE-2	3'–0"xCONT.	16"	4#5 CONT. TOP & BOT. #3 STIRRUPS @ 18" O.C.	THICKENED SLAB EDGE

NOTES:
1. PROVIDE CORNER BARS FOR ALL BARS IN FOOTINGS.
2. ALL BARS ARE BOTTOM BARS U.N.O.

CONCRETE COLUMN SCHEDULE

MARK	SIZE (INCH)	VERTICAL REINFORCING	TIE SPACING	REMARKS
TC1	8x16	4#6	#3 @ 8" O.C.	TYPE A
TC2	12x16	6#6	#3 @ 12" O.C.	TYPE B
TC3	12x22	4#8	#3 @ 12" O.C.	TYPE A
TC4	12x36	8#7	#3 @ 12" O.C.	TYPE C
TC5	12x24	4#8	#3 @ 12" O.C.	TYPE A
TC6	12x18½	6#6	#3 @ 12" O.C.	TYPE B
C1	8x16	4#7	#3 @ 8" O.C.	TYPE A
MC1	8x16	2#6	–	1#6 IN EA. OF TWO GROUT FILLED CELLS

NOTES:
1. ✱ INDICATES PROVIDE CLASS 'B' LAP SPLICES FOR ALL BARS.



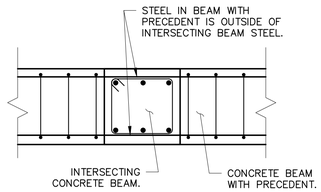
TYPE 'A' TYPE 'B' TYPE 'C'

BEAM SCHEDULE

BEAM NUMBER	ELEV. TOP OF BEAM	SIZE WxH (INCH)	REINFORCING				CLOSED HOOP SIZE AND SPACING	REMARKS
			BOT	TOP	'C'	'E'		
RTB–1	10'–0"	8X16	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–2	VARIES	8X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–3	VARIES	8X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–4	14'–0"	8X24	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–5	11'–0"	8X12	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–6	14'–0¾"	8X16¾	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–7	VARIES	8X12 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–8	14'–0"	8X16	2#5	2#5	–	–	#3 @ 18" O.C.	f'c = 5,000 psi
RTB–9	14'–4"	8X16	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–10	VARIES	8X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–11	19'–0"	12X20	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–12	23'–10"	12X22	2#5	2#5	–	–	#3 @ 18" O.C.	f'c = 5,000 psi
RTB–13	VARIES	8X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–14	13'–1⅜"	8X18⅝	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–15	15'–5⅜"	8X14⅝	2#5	2#5	–	–	#3 @ 18" O.C.	ADD 1#5 CONT. EA. FACE AT MID DEPTH
RTB–16	12'–4"	8X12	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–17	VARIES	8X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–18	VARIES	12X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–19	19'–0"	12X20	2#5	2#5	–	–	#3 @ 18" O.C.	f'c = 5,000 psi
RTB–20	13'–0"	12X20	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–21	VARIES	8X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	T.O. BEAM TO MATCH T.O. ROOF DECK ELEVATION
RTB–22	12'–2"	8X18	2#5	2#5	–	–	#3 @ 18" O.C.	HOLD T.O. BEAM 1½" BELOW JOIST BOT. CHORD
RTB–23	VARIES	8X20 (MIN.)	2#5	2#5	–	–	#3 @ 18" O.C.	SEE ARCH. FOR WALL SLOPE COORDINATION & ELEVATIONS
RTB–24	21'–0⅜"	8X12⅜	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–25	14'–2"	8X18	2#5	2#5	–	–	#3 @ 18" O.C.	HOLD T.O. BEAM 1½" BELOW JOIST BOT. CHORD
RTB–26	13'–4"	8X24	2#6	2#6	–	–	#3 @ 18" O.C.	ADD 2#6 CONT. EA. FACE EVENLY SPACED
RTB–27	VARIES	8X14	2#5	2#5	–	–	#3 @ 18" O.C.	HOLD T.O. BEAM 1½" BELOW JOIST BOT. CHORD
RTB–28	19'–0"	8X12	2#5	2#5	–	–	#3 @ 18" O.C.	f'c = 5,000 psi
RTB–29	23'–10"	8X14	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–30	11'–8"	8X20	2#5	2#5	–	–	#3 @ 18" O.C.	
RTB–31	13'–0"	8X20	2#5	2#5	–	–	#3 @ 18" O.C.	
B–1	10'–0"	8X20	2#5	2#5	–	–	#3 @ 8" O.C.	
B–2	11'–6"	8X24	2#6	2#6	–	–	#3 @ 10" O.C.	
B–3	10'–0"	8X24	2#5	2#5	–	–	#3 @ 10" O.C.	ADD 1#5 CONT. EA. FACE AT MID DEPTH
B–4	11'–4"	8X16	2#7	2#7	–	–	#3 @ 6" O.C.	
B–5	10'–8"	12X20	3#8	2#6	–	–	#3 @ 6" O.C.	
B–6	16'–0"	12X24	2#6	2#8	–	–	#3 @ 10" O.C.	3–SPAN CONT. BEAM
B–7	10'–8"	12X20	2#6	2#6	–	–	#3 @ 6" O.C.	
B–8	9'–4"	8X16	2#5	2#5	–	–	#3 @ 6" O.C.	
B–9	10'–2"	8X16	2#5	2#5	–	–	#3 @ 6" O.C.	ARCH. TO VERIFY ELEVATION
B–10	12'–0"	8X12	2#5	2#5	–	–	#3 @ 4" O.C.	
B–11	14'–0"	8X24	2#6	2#6	–	–	#3 @ 10" O.C.	
B–12	14'–0"	8X24	2#7	2#7	–	–	#3 @ 10" O.C.	TENSION LAP TOP BARS W/ B–15, EXTEND BOT. BARS INTO B–15
B–13	13'–7"	8X19	2#9	2#9	–	–	#3 @ 6" O.C.	ARCH. TO VERIFY ELEVATION
B–14	11'–4"	8X16	2#6	2#6	–	–	#3 @ 6" O.C.	ADD 1#6 CONT. EA. FACE AT MID DEPTH
B–15	14'–0"	8X40	2#7	2#7	–	–	#3 @ 16" O.C.	TENSION LAP TOP BARS W/ B–12, EXTEND BOT. BARS FROM B–12

BEAM SCHEDULE NOTES:

1. FOR BEAMS OVER 36" DEEP, PROVIDE #4 BARS AT 6" O.C. ON EACH FACE OF BEAM, U.N.O. PROVIDE CLASS "B" LAP SPLICES AS REQUIRED.
2. TOP STEEL CONTINUOUS UNLESS OTHERWISE NOTED.
3. IF BEAM OCCURS ACROSS A STEP LOCATION, DROP TOP STEEL FOR SMALLER BEAM HEIGHT.
4. TOP OF BEAM ELEVATION IS IN REFERENCE TO DEEPEST PORTION OF BEAM.
5. TOP STEEL IN CONTINUOUS BEAMS SHALL TAKE PRECEDENT OVER INTERSECTING SINGLE–SPAN BEAMS AT INTERSECTIONS.
6. WHERE SPANNING BEAMS AND TIEBEAMS WILL SUPPORT THE BEARING END OF BAR JOIST FRAMING OR JUST SUBSTITUTE OUTRIGGERS, BEAM SHALL BE POCKETED FOR JOIST BEARING AND BEAM TOP STEEL SHALL BE HELD BELOW POCKET. STEEL DETAILER TO ADJUST STIRRUP DIMENSIONS ACCORDINGLY.



BEAM STEEL PRECEDENT DETAIL
N.T.S.

Lars W Young Inc

ANTHONY R. WILSON LICENSE No. 79451 STATE OF FLORIDA PROFESSIONAL ENGINEER
10/1/2025
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North Collier Fire Control & Rescue District Station 49

COCOHATCHEE ROAD NAPLES FLORIDA 34110
FOR: NORTH COLLIER FIRE RESCUE DISTRICT

SCHEDULES

Florida Registration # AR0016755

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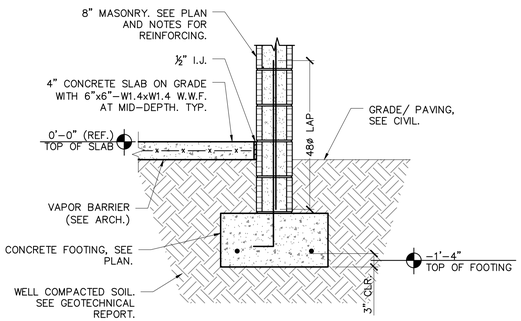
S4.0



SECTION 1 — S5.0
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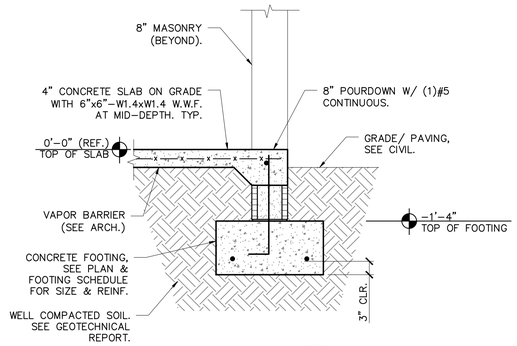

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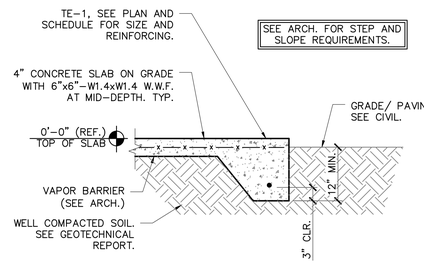
SCALE: 3/4"= 1'–0"



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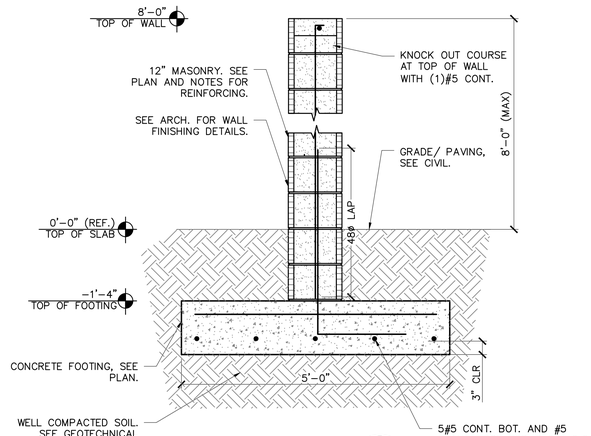

SECTION 4 — S5.0
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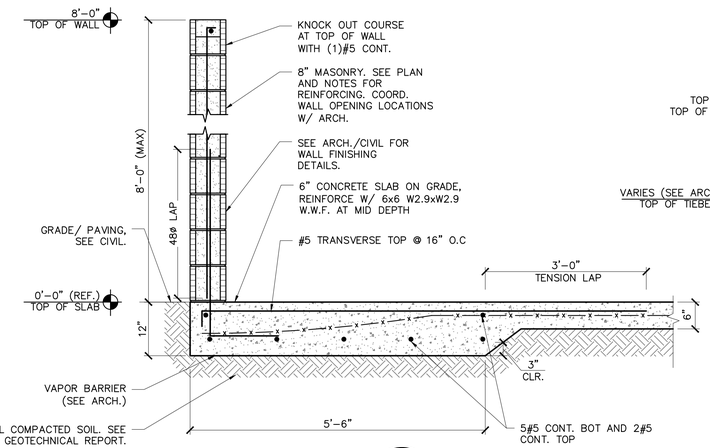

SECTION 5 — S5.0
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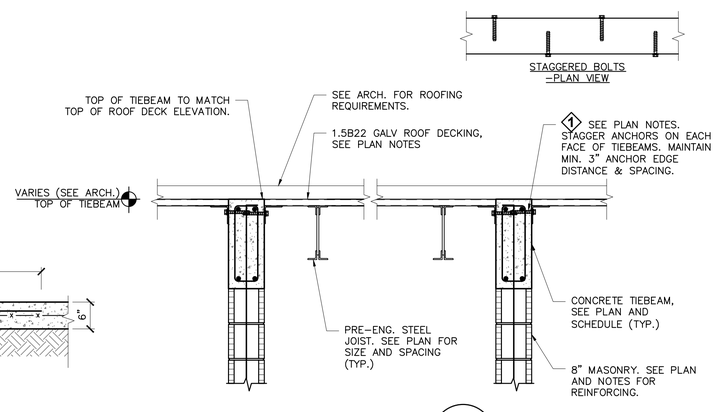
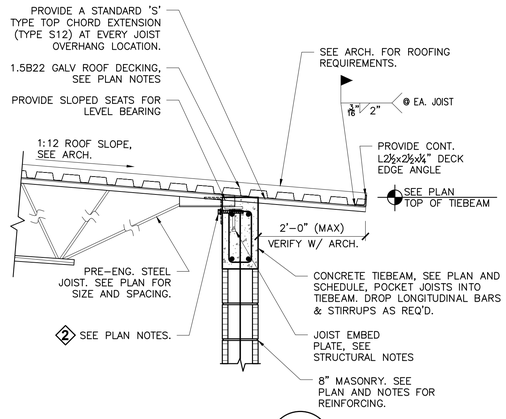
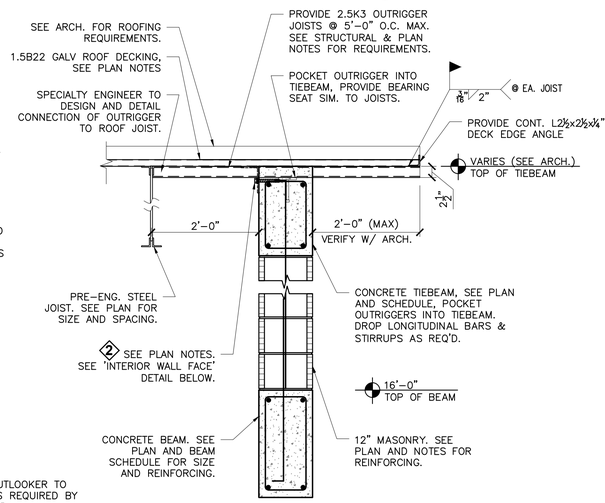
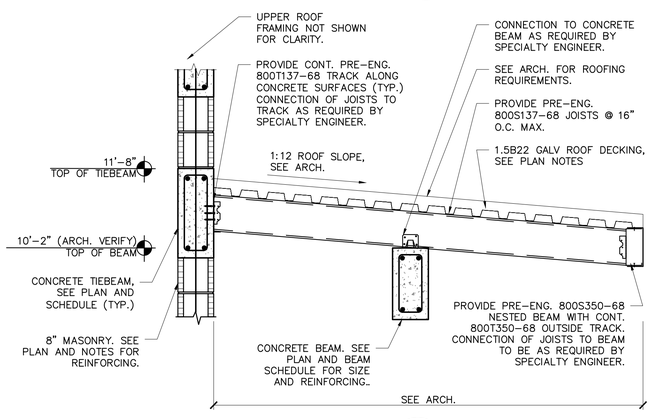


SECTION 6 — S5.0
SCALE: 3/4"= 1'–0"

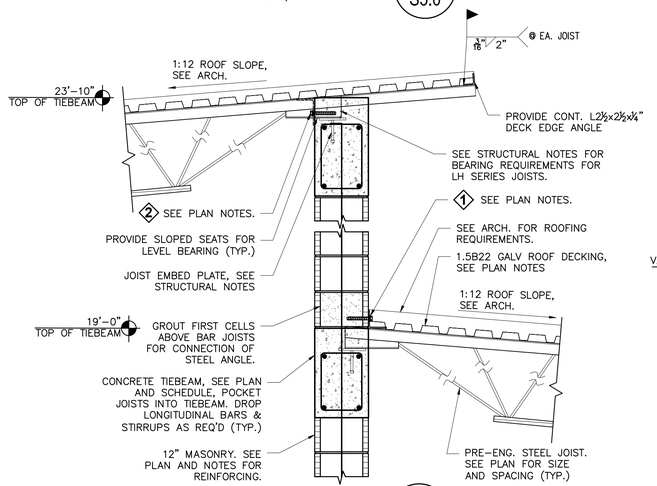


SECTION 7 — S5.0
SCALE: 3/4"= 1'–0"



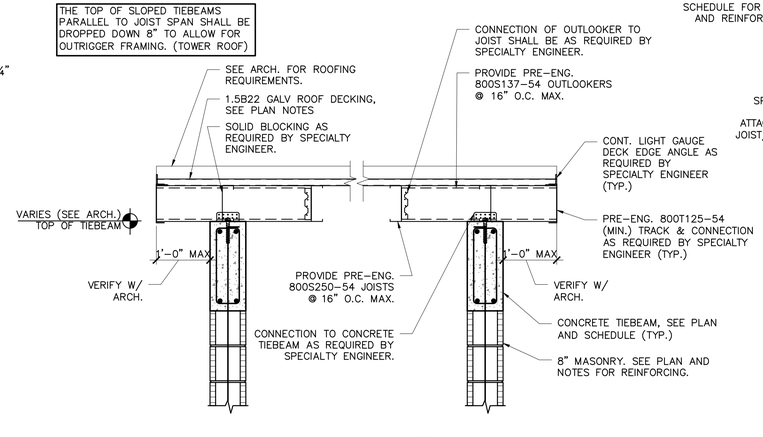


SECTION 9 — S5.0
SCALE: 3/4"= 1'–0"

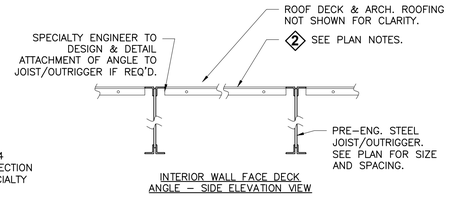


SECTION 10 — S5.0
SCALE: 3/4"= 1'–0"

THE TOP OF SLOPED TIEBEAMS PARALLEL TO JOIST SPAN SHALL BE DROPPED DOWN 8" TO ALLOW FOR OUTRIGGER FRAMING. (TOWER ROOF)



SECTION 11 — S5.0
SCALE: 3/4"= 1'–0"



SECTION 8 — S5.0
SCALE: 3/4"= 1'–0"

Lars W Young Inc

ANTHONY R. WILSON LICENSE No. 79451 STATE OF FLORIDA PROFESSIONAL ENGINEER
10/1/2025
ENGINEER OF RECORD
ANTHONY R. WILSON
FL P.E. # 79451

Wilson Structural

6931 PROFESSIONAL PKWY. WEST
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(941) 907-4789 FAX(941) 907-0576

4230 SOUTH MacDILL AVE
SUITE H TAMPA, FL 33611
(813) 536-3476

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS SHALL COMPLY WITH ALL APPLICABLE MINIMUM BUILDING CODES.

FILE: 24DW-0502

FIRM REGISTRY #36167

No.	Description	Date

Project #
Date: 30 September, 2025
Drawn: HP
Checked: JK
Scale 1/8 =1"

North Collier Fire Control & Rescue District Station 49

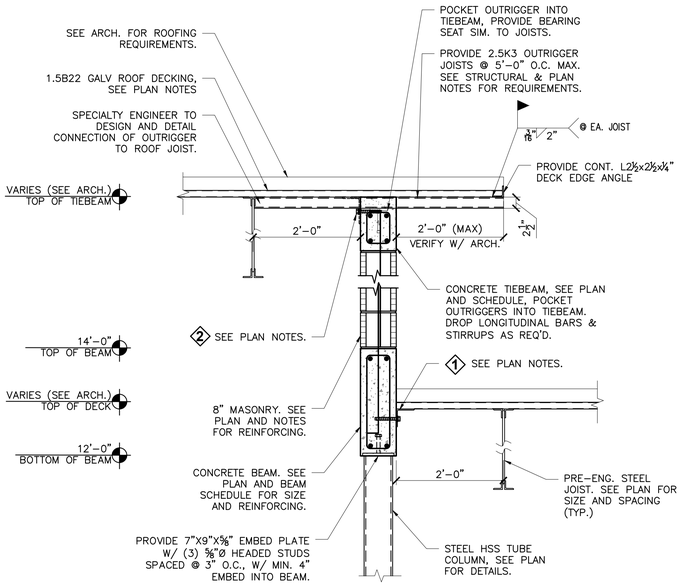
COCOHATCHEE ROAD NAPLES FLORIDA 34110
FOR: NORTH COLLIER FIRE RESCUE DISTRICT

SECTIONS

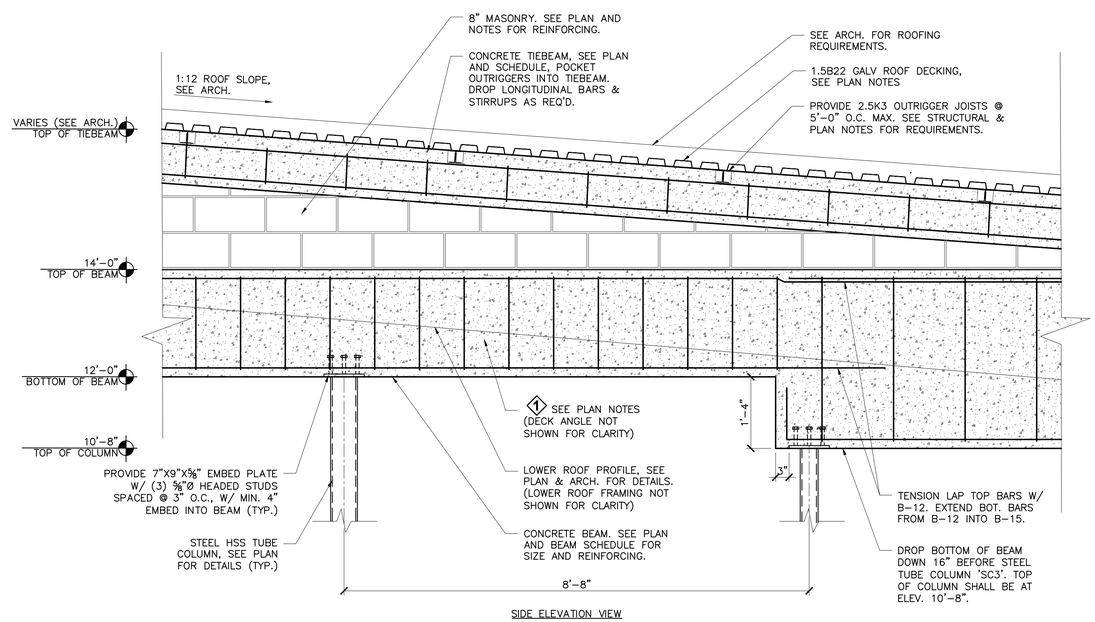
Florida Registration # AR0016755

Architecture
6961 Sable Ridge Ln
Naples, FL 34109
phone: 239.591.8899
lwyarch@gmail.com

S5.0

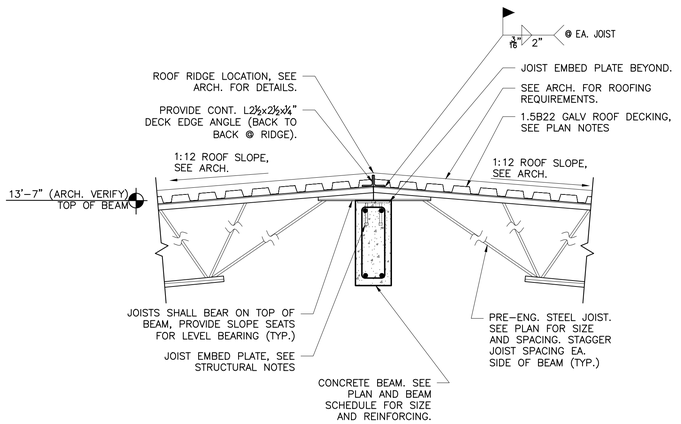


SECTION 12
SCALE: 3/4"= 1'-0"
S5.1

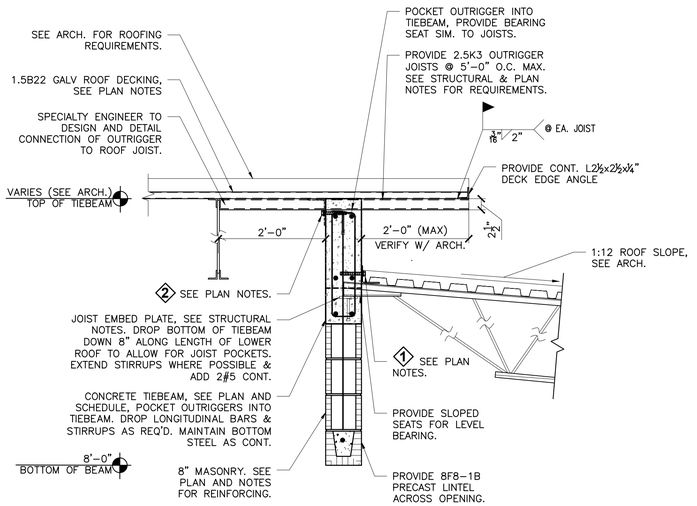


SIDE ELEVATION VIEW

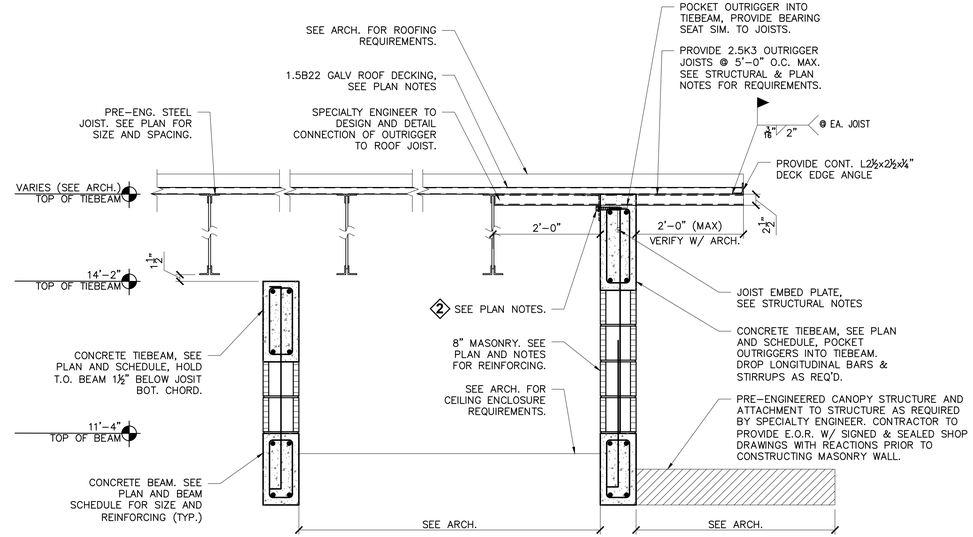
SECTION 13
SCALE: 3/4"= 1'-0"
S5.1



SECTION 14
SCALE: 3/4"= 1'-0"
S5.1



SECTION 15
SCALE: 3/4"= 1'-0"
S5.1



SECTION 16
SCALE: 3/4"= 1'-0"
S5.1

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S5.1